FRACTAL DIMENSIONS OF A POROUS CONCRETE 
AND ITS EFFECT ON THE CONCRETE’S STRENGTH

Chun-Hui He¹, Chao Liu¹,²

¹School of Civil Engineering, Xi'an University of Architecture and Technology, Xi'an, China
²School of Science, Xi'an University of Architecture and Technology, Xi'an, China

Abstract. All mechanical properties of a porous medium depend upon its fractal dimensions, however, how to measure the fractal dimensions is still an open issue. This paper adopts the two-scale fractal theory to calculate fast and effectively the fractal dimensions of a porous concrete. Of the concrete’s properties that have been fascinating engineers and scientists, by far the most perplexing is the effects of its porosity and pore size on concrete’s strength. Though there were many ad hoc empirical formulae for predicting the strength, much deviation arose for practical applications. Here a dimensionless model and the fractal theory are adopted to insight theoretically into the effects, and for the first time ever, some physically relative and mathematically reliable formulations are proposed. Additionally nano/micro particles’ size and distribution can also be used for theoretical prediction of the concrete’s strength, it shows that the boundary-induced force occurs when the particles tend to micro/nanoscales. The present theory sheds new light on the optimal design of various functional concretes.

Key words: Two-scale fractal, Geometric potential, Dimensionless analysis, Hall-Petch effect, Porosity

1. INTRODUCTION

A porous medium always behaves extremely attractively compared to its continuum partner, the latter is focus of the continuum mechanics, which has matured into a fully-fledged theory, and has laid the foundation for the mechanical engineering, however, there is no universal theory for porous problems. Xue and Liu [1] found that a porous medium with a hierarchical structure has an excellent heat insulation. Xo, et al. [2] revealed the mechanism of heat prevention for cocoon-like hierarchy. Xue, et al. [3] further elucidated cocoon's biomechanism using the fractal theory. Hierarchical porous materials are now
widely used for high-rate electrochemical capacitive energy storage [4], supercapacitors [5] and energy harvesting [6,7]. Nano/micro scale porous membranes have extremely high permeability and extremely small pressure drop [8-11], the diffusion process in a porous medium (e.g. water) has attracted much attention in the academic community, because the seemingly stochastic diffusion process is actually deterministic in a fractal space, making the impossible possible [12,13,14]. The vibrating process in air can also be considered in a fractal space, and a new discipline, the fractal vibration theory, has been skyrocketing [15,16,17]. The fractal theory and the artificial intelligent have been successfully applied to investigate the hardness properties of tool steel alloys [18,19,20].

Some phenomena arising in porous media cannot be explained by continuum mechanics, where the smooth space is the footstone. This paper focuses on the most commonly used porous material on the Earth, that is the concrete [21,22,23], using the fractal theory [24].

2. FRACTAL DIMENSIONS

All mechanical properties of a continuum medium are relative to its dimensions, for example, its volume scales with the cube of the measured size

\[ V \propto r^3 \]  \hspace{1cm} (1)

where \( V \) is the volume, \( r \) is the measured size. Similarly for a porous medium, its volume can be written as

\[ V \propto r^\alpha \]  \hspace{1cm} (2)

where \( \alpha \) is the fractal dimensions. In a fractal space with a fractal dimensionality \( \alpha \), the volume is a measurement of the measured size. The relation of the fractal dimensionality \( \alpha \) and the volume can be expressed in Eq. (2). When \( \alpha=3 \), it becomes a continuum, and when \( \alpha=0 \), it is an empty pore.

Zuo and Liu elucidated that the mechanical and electrical properties of a composite depend upon the fractal dimensions [25], Mandelbrot, et al. revealed that fracture property of metals can be effectively explained by the fractal dimensions [26], Babič, et al. elucidated that the fractal dimensions are relative to the material’s surface characteristics and mechanical property [18,19,20]. However how to calculate the value of the fractal dimensions is a difficult problem, and mathematicians and engineers will be captivated by an effective and reliable measurement.

There are many methods to calculate fractal dimensions, among which the Hausdorff dimensions are the most used one, its definition is [24, 26]

\[ \alpha = \frac{\ln N}{\ln r} \]  \hspace{1cm} (3)

where \( N \) is new measured units when we measure the fractal pattern using a reduced \( 1/r \) scale. Taking the Cantor set as an example, when we use a reduced 1/3 scale, we find two new units, \( N=2 \), so \( \alpha=\ln2/\ln3 \).

But for a porous medium, we might know only its porosity, then how to calculate its fractal dimensions? We consider a Sierpiński-like porous area as shown in Fig. 1. When the porosity is zero, the area is two dimensional; while when the porosity equals to one, the area
is zero dimensional, so it is obvious that 0<\alpha<2. The Sierpinski carpet is a pure mathematical concept, the first cascade is similar to Fig. 1, however, each unit can continue iteration to form a Hausdorff dimensions of \alpha=\ln 8/\ln 3 when C=L/3.

Feng, et al. [27] suggested the following formulation to calculate its fractal dimensions

\[
\alpha = \frac{\ln(L^2 - C^2)}{\ln L}
\]

where \( L^2 \) and \( C^2 \) are the areas of the measured unit and the porosity, respectively.

Fig. 1 A Sierpinski-like porous area

According to the definition of Eq. (4), we have

\[
\lim_{C \to 0} \alpha = \lim_{C \to 0} \frac{\ln(L^2 - C^2)}{\ln L} = 2
\]

and

\[
\lim_{C \to L} \alpha = \lim_{C \to L} \frac{\ln(L^2 - C^2)}{\ln L} \rightarrow -\infty
\]

Though Eq. (5) meets the continuum assumption for a continuous medium, Eq. (6) is not physically inconsistent. Kong [28] suggested the following modified one

\[
\alpha = \frac{\ln(L^2 / C^2 - 1)}{\ln(L / C)}
\]

It is interesting to note that most natural materials have fractal dimensions closed to 1.618, the golden mean [29,30]. The fractal dimensions are also the key factor affecting a porous concrete’s properties, Rieu and Sposito suggested the following formulation [31]:

\[
\varphi = 1 - \left( \frac{r_{\text{min}}}{r_{\text{max}}} \right)^{3-\alpha_{\text{max}}}
\]
where $\varphi$ is the porosity, $r_{\text{min}}$ and $r_{\text{max}}$ are the largest and smallest pore radiiuses, respectively, $\alpha_{\text{pore}}$ is the fractal dimensions of the pore space.

Yu [32] pointed out that Eq. (8) is physically inconsistent, because for a continuum medium, we have $\alpha_{\text{pore}} = 0$ and $\varphi = 0$; on the other hand, for the full porosity, $\alpha_{\text{pore}} = 3$ and $\varphi = 1$. For the both cases, Eq. (8) gives wrong results. Yu suggested that following one [32]:

$$\varphi = c \left( \frac{r_{\text{min}}}{r_{\text{max}}} \right)^{3-\alpha_{\text{pore}}} \quad (9)$$

For a porous concrete, $c = 1$, and Eq. (9) becomes

$$\varphi = \left( \frac{r_{\text{min}}}{r_{\text{max}}} \right)^{3-\alpha_{\text{pore}}} \quad (10)$$

Eq. (10) is physically consistent and mathematically reliable. But in practical applications, we have difficulty in determining $r_{\text{min}}$ and $r_{\text{max}}$, and its fractal dimensions cannot be calculated through the porosity.

In this paper we adopt the two-scale fractal dimensions [33-35], which uses two scales, $L$ and $C$, to measure the area for Fig. 1. When we use the scale of $L$, any pores with sizes less than $L$ are ignored, so the area is $L^2$ with a two-dimensional property; when we measure it using a scale of $C$, its area becomes $L^2-C^2$. According to the definition of the two-scale fractal dimensions [33-35], we have

$$\frac{2}{\alpha} = \frac{L^2}{L^2-C^2} \quad (11)$$

or

$$\alpha = \frac{2(L^2-C^2)}{L^2} \quad (12)$$

For a porous concrete, when using a large scale, we can obtain its volume, $V$, with a three-dimensional property; while if we use a micro scale, the porous structure can be found, and the two-scale fractal dimensions for the concrete can be calculated through the following relationship [33]:

$$\frac{V - V_{\text{pore}}}{V} = \frac{\alpha}{3} \quad (13)$$

where $V_{\text{pore}}$ is the total volume for pores. The porosity can be expressed as

$$\varphi = \frac{V_{\text{pore}}}{V} \quad (14)$$

So the two-scale fractal dimensions of the porous concrete can be calculated as

$$\alpha = 3(1-\varphi) \quad (15)$$
The fractal dimensions for the porous space in the concrete can be expressed as

\[ \alpha_{\text{pore}} = 3 - \alpha = 3\phi \]  

(16)

The two-scale theory has become an effective tool to various discontinuous problems [36-40].

3. CONCRETE’S STRENGTH VS. FRACTAL DIMENSIONS

Concrete is a porous material, and the porosity and the pore size significantly affect the concrete’s properties, especially its strength. There are many empirical formulae to express the relationship between the strength and its pore structure. The most famous one is [41, 42, 43]

\[ F = F_0 (1 - \phi)^m \]  

(17)

where \( F \) is the concrete’s strength, \( F_0 \) is the strength of its continuum partner with zero porosity, \( m \) is an empiric constant, \( \phi \) is the porosity.

Eq. (17) reflects only the effect of porosity, and the pore size is not considered. Kumar and Bhattacharjee suggested the following one [43]

\[ F = F_0 K \left( \frac{1 - \phi}{r^{3/2}} \right) \]  

(18)

where \( K \) is a constant, \( r \) is the pores’ average radius.

In Eq. (17), the physical understanding of the parameter \( m \) lacks, and there is no practical criterion for choosing its value. Though Eq. (18) considers the effects of porosity and pores’ size, the parameter \( K \) also lacks its physical meaning. The main problem of Eq. (18) is that the parameter \( K \) is dimension-related and physically irrelative. We re-write Eq. (18) in the form

\[ \frac{F}{F_0} = K \left( \frac{1 - \phi}{r^{3/2}} \right) \]  

(19)

When the porosity \( \phi \) tends to zero, we have \( r=0 \), the concrete becomes a continuum medium and Eq. (19) implies that \( F/F_0 \) becomes infinitely large instead of \( F/F_0=1 \), so this is physically irrelative. Furthermore the left side of Eq. (19) is dimensionless, so the dimension of \( K \) has to be \( m^{-1/2} \). The value of \( K \) is different if \( r \) uses different dimensions, e.g., nanometer or micrometer. In order to resolve this apparent contradiction, the dimensionless analysis [44,45] can be powerfully applied, which is the central dogma of complex problems. Using the dimensionless analysis, Estrada-Diaz, et al. [44] found a useful mathematical formulation for electrospinning, He, et al. [45] established a bond stress-slip model for 3-D printed concretes, and Kong [28] found a totally new friction law for porous fabrics. According to the dimensionless analysis, Eq. (15) can be modified as

\[ \frac{F}{F_0} = K_{\text{dimensionless}}(1 - \phi)^{\alpha} \left( \frac{r_w}{r_{\text{ave}}} \right)^{\alpha} \]  

(20)
where $K_{\text{dimensionless}}$ is a material constant, $a$ and $b$ are constants to be determined later, $r_m$ is the average radius, $r_0$ is the porosity size of a reference pore, it can be the minimal/maximal porosity size.

Eq. (20) is a dimensionless formula, so it is physically relative and mathematically reliable. In order to determine the value of $a$, we write down the concrete's strength in the form

$$F = \sigma A$$  \hspace{1cm} (21)

where $\sigma$ is the stress, $A$ is the contacted section area. Considering the porosity, the whole volume can be calculated as

$$V = V_0 (1 - \varphi)$$ \hspace{1cm} (22)

where $V_0$ is the sample's total volume.

The section area scales approximately with

$$A \varpropto V^{2/3}$$ \hspace{1cm} (23)

So we have

$$A = KV^{2/3} = K_{\text{dimensionless}}V_0^{2/3} (1 - \varphi)^{2/3}$$ \hspace{1cm} (24)

According to the above relationships, we obtain

$$F = K_{\text{dimensionless}}\sigma V_0^{2/3} (1 - \varphi)^{2/3} = K_{\text{dimensionless}}F_0 (1 - \varphi)^{2/3}$$ \hspace{1cm} (25)

where $F_0$ is the strength with zero porosity.

If we have the section's area porosity, $\rho$, the actual area can be written as

$$A = A_0 (1 - p)$$ \hspace{1cm} (26)

where $A_0$ is the section area when $p=0$.

Eq. (21) becomes

$$F = \sigma A_0 (1 - p)$$ \hspace{1cm} (27)

According to Eq. (23), the area porosity and the volume porosity have the following approximate relationship

$$(1 - p) \varpropto (1 - \varphi)^{2/3}$$ \hspace{1cm} (28)

After a simple calculation, we have

$$F = K_{\text{dimensionless}}\sigma A_0 (1 - p) = K_{\text{dimensionless}}F_0 (1 - \varphi)^{2/3}$$ \hspace{1cm} (29)

which is the same as Eq. (25).

According to the above analysis, the value of $m$ in Eq. (17) and $a$ in Eq. (20) should be approximately $2/3$. Eq. (17) should be corrected as

$$F = K_{\text{dimensionless}}F_0 (1 - \varphi)^{2/3}$$ \hspace{1cm} (30)
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In Ref. [42], $m=8.15$, this large deviation is due to the ignorance of the effect of pores’ size. When the pores' size tends to micro/nano scales, the size effect [46] becomes enormous. Now we correct Eq. (20) as

$$\frac{F}{F_0} = K_{\text{dimensionless}} (1 - \varphi)^{2/3} \left( \frac{R_0}{r_m} \right)^b$$

(31)

To understand the parameter $b$, we explain the size effect [46] through the geometric potential theory [47]. When the pores' size tends to nano/micro scales, high surface energy (geometric potential) [47] can be produced. The geometric potential theory assumes that any surface produces a boundary-induced force, and it can explain many complex phenomena, for example, Fangzhu’s absorption of water molecules from the air [48,49], the nanofiber’s wetting [47], and the cell orientation [50].

In order to use the geometric potential theory [47], we modify Eq. (31) in the form

$$\frac{F}{F_0} = K_{\text{particle}} (1 - \varphi)^{2/3} \left( \frac{R_0}{R_m} \right)^b$$

(32)

where $K_{\text{particle}}$ is a geometrical parameter, $R_m$ is the average radius of the particles in the concrete, $R_0$ is the reference size. The geometric potential of particles can produce a surface force [47]:

$$f = -\frac{d\Pi}{dR_m}$$

(33)

where $\Pi$ is the geometric potential produced by the particles. Generally, it can be expressed as [47]

$$\Pi \propto \frac{1}{(R_m)^\beta}$$

(34)

where $\beta$ is the geometrical parameter. For a sphere like the Sun, $\beta=1$, which leads to Newton's gravity.

The concrete's strength due to nano/micro particles can be expressed as

$$F \propto K_{\text{particle}} \left( \frac{R_0}{R_m} \right)^b$$

(35)

where $b=\beta+1$. Generally $b=1/2$ for the qualitative analysis as that in Hall-Petch effect [51].

$$\frac{F}{F_0} = K_{\text{particle}} (1 - \varphi)^{2/3} \left( \frac{R_0}{R_m} \right)^{1/2}$$

(36)

Generally we have $R_m$ scales with $r_m$, so Eq. (36) can be expressed as Model I:

$$\frac{F}{F_0} = K_{\text{dimensionless}} (1 - \varphi)^{2/3} \left( \frac{r_0}{r_m} \right)^{1/2}$$

(37)
Using the experimental data given in Ref. [43], \( K_{\text{dimensionless}} \) and \( r_0 \) in Eq. (37) can be identified, and finally for the studied concrete of Ref. [43], we have

\[
\frac{F}{F_0} = 52.9(1-\varphi)^{2/3} \left(\frac{13.64}{r_m}\right)^{1/2}
\]  

(38)

Fig. 2 shows the comparison between the theoretical prediction of Eq. (38) and experimental data given in Ref. [43], and a relative agreement is seen. The deviation arises in various factors, the main factor is the pore size distribution because, in our theory analysis, only average pore size is considered.

As discussed above, \( b=1/2 \) is only used for the qualitative analysis. To understand the parameter \( b \), the fractal theory has to be adopted. As the concrete’s strength is reflected by the contacted area, in a fractal space, the area and the volume have the following scaling relationship:

\[
A \propto V^{(\alpha-1)/\alpha}
\]  

(39)

When \( \alpha=3 \), we have the well-known 2/3 scaling law:

\[
A \propto V^{2/3}
\]  

(40)

For a 4-dimensional space, we have

\[
A \propto V^{3/4}
\]  

(41)

This 3/4 scaling law plays an important role in life science [52]. In a fractal space, Eq. (39) holds exactly, and Eq. (23) is approximate one, so Eq. (37) can be further improved as Model II:
Fractal Dimensions of a Porous Concrete and its Effect on the Concrete's Strength

\[ \frac{F}{F_0} = K_\alpha (1 - \phi)^{(a - 3/\alpha) \phi \alpha} \left( \frac{r_0}{r_m} \right)^{1/2} \]  \hfill (42)

where \( K_\alpha \) is a geometric parameter, \( a = 3(1 - \phi) \).

Using the experimental data given in Ref. [43] to determine \( K_\alpha \) and \( r_0 \) in Eq. (42), we have

\[ \frac{F}{F_0} = 44.0(1 - \phi)^{(a - 4/\alpha) \phi \alpha} \left( \frac{19.47}{r_m} \right)^{1/2} \]  \hfill (43)

Fig. 3 shows the comparison between the theoretical prediction of Eq. (43) and experimental data given in Ref. [43].

We write the concrete's strength in the form

\[ F = K_{\text{particle}} \sigma V^c \left( \frac{1}{R_m} \right)^d = K_{\text{particle}} F_0 V_0^{(c-2)/3} (1 - \phi)^\gamma \left( \frac{1}{R_m} \right)^d \]  \hfill (44)

where \( K_{\text{particle}} \) is a geometrical constant, \( c \) and \( d \) are constants. According to the dimensionless analysis, the following equation should be satisfied:

\[ 3(c - \frac{2}{3}) - d = 0 \]  \hfill (45)
So we have Model III:

\[
\frac{F}{F_0} = K_{\text{particle}} V_0^{c-2/3} (1 - \varphi)^{\frac{1}{c}} \left( \frac{1}{R_m} \right)^{(3c-2)}, c > 2/3
\]  \hspace{1cm} (46)

and

\[
\frac{F}{F_0} = K_{\text{pore}} V_0^{c-2/3} (1 - \varphi)^{\frac{1}{c}} \left( \frac{1}{R_m} \right)^{(3c-2)}, c > 2/3
\]  \hspace{1cm} (47)

Eq. (46) or Eq. (47) reveals that the concrete's strength depends also upon its volume. Using the experimental data given in Ref. [43], we have approximately the following formulation:

\[
\frac{F}{F_0} = 136.2(1 - \varphi)^{0.708} \left( \frac{1}{R_m} \right)^{0.394}
\]  \hspace{1cm} (48)

Fig. 4 shows the comparison between the theoretical prediction of Eq. (48) and experimental data given in Ref. [43].

**Fig. 4** Comparison between the theoretical prediction of Eq. (47) and experimental data given in Ref. [43].
Table 1 Comparison of our two models with experimental data [43]

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Fig. 4 and Table 1 show the deviation of Eq. (48) becomes much less than those of Eq. (38) and Eq. (43), showing the reliability of our theoretical model given in Eq. (47).

4. DISCUSSION AND CONCLUSIONS

If data for the porosity size is segmented, the following one can be considered:

\[ F = K_F (1 - \varphi)^{2/3} \left( \sum_{i=1}^{N} a_i \left( \frac{r_i}{r_{im}} \right)^{\varphi} \right)^{1/3} \]

(49)
where $r_{\text{im}}$ is the $i$-th segment’s average radius, $r_{0}$ is the $i$-th segment’s reference radius, which can be the segment’s largest radius, $a_{i}$ is the weighting factor.

If the pores are distributed continuously, then we have

$$ F = K F_{0} (1 - \varphi)^{2/3} \int_{r_{\text{im}}}^{r_{0}} \frac{(r_{0} - r)^{3}}{r^{2}} \varphi \, dr $$

(50)

This paper suggests some conformable formulations for estimating the strength of a porous concrete, making it applicable to various cases, and shedding new light on the optimal design of the porous concrete with a given strength. In our theory, particles’ size distribution might be useful for practical applications.

Though our mathematical dimensionless model is mathematically correct and physically relevant, experimental verification is very much needed in future, and the fractal-fractional calculus [53,54] can be used for dynamical analysis of the porous concrete.

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